Appendix F

Hydraulics Report



Final Hydraulics Report

Kern River Intake Replacement Project

Prepared for Water Works Engineers, LLC March 26, 2024

→ The Power of Commitment



Project n	ame	Kern River Hydraulic Analysis									
Documer	nt title	Final Hydraulics Report Kern River Intake Replacement Project									
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1. Introduction

The community of Kernville, in Kern County, is planning to construct a new water intake structure on the Kern River just upstream of the Kernville Road bridge (see Figure 1) and Water Works Engineers is providing the design of the intake structure. The project is within a Federal Emergency Management Agency (FEMA) Regulatory Floodway and as such requires a No-Rise Certificate for the construction of the intake structure. Water Works Engineers engaged GHD to conduct a study to estimate the impact of the proposed intake structure on the flood elevations in the vicinity of the project. The methods and results of that study are presented in this report.



Figure 1 Project vicinity map

1.1 Purpose of this report

The purpose of this report is to summarize the methods and results from the hydraulic analysis performed by GHD intended to assess the flood impacts of the proposed intake structure replacement project. The analysis evaluated the impacts of the intake structure on the 100-year water surface elevations within the Kern River in the vicinity of the intake structure.

1.2 Scope and limitations

This report has been prepared by GHD for Water Works Engineers, LLC and may only be used and relied on by Water Works Engineers, LLC for the purpose agreed between GHD and Water Works Engineers, LLC as set out in section 1.1 of this report.

GHD otherwise disclaims responsibility to any person other than Water Works Engineers, LLC arising in connection with this report. GHD also excludes implied warranties and conditions, to the extent legally permissible.

The services undertaken by GHD in connection with preparing this report were limited to those specifically detailed in the report and are subject to the scope limitations set out in the report.

The opinions, conclusions and any recommendations in this report are based on conditions encountered and information reviewed at the date of preparation of the report. GHD has no responsibility or obligation to update this report to account for events or changes occurring subsequent to the date that the report was prepared.

The opinions, conclusions and any recommendations in this report are based on assumptions made by GHD described in this report (refer section 1.3 of this report). GHD disclaims liability arising from any of the assumptions being incorrect.

GHD has prepared the hydraulic model ("Model") for, and for the benefit and sole use of, Water Works Engineers, LLC to support this study and must not be used for any other purpose or by any other person.

The Model is a representation only and does not reflect reality in every aspect. The Model contains simplified assumptions to derive a modelled outcome. The actual variables will inevitably be different to those used to prepare the Model. Accordingly, the outputs of the Model cannot be relied upon to represent actual conditions without due consideration of the inherent and expected inaccuracies. Such considerations are beyond GHD's scope.

The information, data, and assumptions ("Inputs") used as inputs into the Model are from publicly available sources or provided by or on behalf of the Water Works Engineers, LLC, (including possibly through stakeholder engagements). GHD has not independently verified or checked Inputs beyond its agreed scope of work. GHD's scope of work does not include review or update of the Model as further Inputs becomes available.

The Model is limited by the mathematical rules and assumptions that are set out in the Report or included in the Model and by the software environment in which the Model is developed.

The Model is a customised model and not intended to be amended in any form or extracted to other software for amending. Any change made to the Model, other than by GHD, is undertaken on the express understanding that GHD is not responsible, and has no liability, for the changed Model including any outputs.

GHD has prepared this report on the basis of information provided by Water Works Engineers, LLC and others who provided information to GHD (including Government authorities), which GHD has not independently verified or checked beyond the agreed scope of work. GHD does not accept liability in connection with such unverified information, including errors and omissions in the report which were caused by errors or omissions in that information.

Accessibility of documents

If this report is required to be accessible in any other format, this can be provided by GHD upon request and at an additional cost if necessary.

1.3 Assumptions

The information included in this report is based on the following assumptions:

- The project topographic data is representative of the actual existing conditions at the site.
- Cross-sections were surveyed from river station 9,391 to 12,095, however the surveyed bathymetry was
 incomplete due to fast flowing water in the channel. Where data was unavailable, the bathymetry was estimated
 by a combination of interpolation and estimation from the HEC-2 data.
- Detailed survey information for the Kernville Road bridge was not provided and the top of deck elevation was interpolated between surveyed high points at either end of the bridge. The deck thickness and corresponding low chord elevation were taken from the HEC-2 model.

2. Sources of data

2.1 FEMA

The existing effective FEMA hydraulic model for the Kern River is a HEC-2 model and the model input and output were provided to GHD in PDF format, referenced to the National Geodetic Vertical Datum of 1929 (NGVD 29). The FEMA Flood Insurance Study (FIS) for Kern County specifies the average vertical datum conversion factor for the Kern River at Kernville as adding 3.3 feet to elevations to convert from the NGVD 29 to the North American Vertical Datum of 1988 (NAVD 88) (FEMA, 2021). The National Flood Hazard Layer (NFHL) GIS data was obtained from FEMA and used to map the river centerline, floodway, and the special flood hazard areas that are represented on the Flood Insurance Rate Map (FIRM), which is included in Appendix A. The FIS was referenced for the 1% Annual Chance (100-year) peak discharge and associated flood water surface elevations at the cross-sections shown on the FIRM. The HEC-2 data was used for cross-section information and bridge geometry. The FIS 100-year peak discharge of 69,000 cubic feet per second (cfs) was used as the design flow.

2.2 Topographic survey

Topographic survey data was collected for the design of the project and the hydraulic analysis presented herein by Fugro in 2023. The data included a LiDAR digital elevation model with 10-foot grid spacing extending beyond the limits of the flood inundation extents and surveyed channel cross-sections were used in the updated hydraulic models to better represent the current existing conditions in the area. Channel cross-sections were surveyed approximately every two hundred feet along the study reach, although complete cross-sections were unable to be surveyed due to fast flowing water in the channel. Where cross-section data was not obtained, the channel bottom was approximated by interpolation or estimation from the HEC-2 data. The horizontal and vertical datums used were:

- Horizontal Datum: North American Datum of 1983 (NAD 83)
- State Plane Coordinate System: California State Plane, Zone 5
- Vertical Datum: North American Vertical Datum of 1988 (NAVD 88)
- Units: U.S. Foot

2.3 Proposed improvements

The proposed improvements include a new intake structure and an associated electrical equipment stand. The proposed intake structure has been designed such that it does not project out from the existing riverbank and is assumed to cause zero obstruction to flow. The electrical equipment stand is located out of the main channel but within the floodplain and the obstructed area attributed to it was calculated using the submerged cross-sectional area

of the structure perpendicular to the flow of the Kern River. The obstructed area of the electrical stand was calculated to be 23.5 square feet.

3. Hydraulic analysis

A steady flow, one-dimensional (1-D) hydraulic analysis was performed using the United States Army Corps of Engineers Hydraulic Engineering Center River Analysis System (HEC-RAS) software (Version 6.3.1).

3.1 Duplicate Effective Model

A Duplicate Effective Model (DEM) was developed for the Kern River in the vicinity of the project using data from the HEC-2 model for the cross-sections and the Kernville Road bridge. A hydraulic work map showing the FEMA FIS cross-sections, HEC-2 cross-sections, floodway, and special flood hazard areas is shown in Figure 1 of Appendix B. The DEM was created from the HEC-2 data according to the following methodology:

- Cross-section and bridge information was compiled from the HEC-2 data and converted from the NGVD 29 to NAVD 88 adding the average conversion factor of 3.3 feet as specified in the FIS. The cross-section information included station and elevation data, bank stations, and downstream reach lengths as well as Manning's roughness coefficients ("n" values), contraction, and expansion coefficients. The bridge information included bridge deck, pier, and abutment geometry as well as total loss, pier, and discharge coefficients.
- A non-georeferenced 1-D river reach was created in HEC-RAS and the compiled HEC-2 information from FIS
 cross-section J to FIS cross-section M was added to the model geometry.
- Roughness coefficients (Manning's "n" values) varied from 0.04 to 0.041 in the main channel and 0.04 to 0.056 in the overbanks.

3.2 Existing Conditions Model

An Existing Conditions Model (ECM) was developed to incorporate updates to the channel geometry based on the survey data collected for the project. The ECM was created according to the following methodology:

- The topographic survey was imported into to HEC-RAS and a 1-D river reach was created following the Kern River centerline from the FIRM extending from FIS cross-section J (station 8,792) at the downstream end to FIS cross-section M (station 12,095) at the upstream end.
- Cross-sections were added at the approximate HEC-2 cross-section locations from FIS cross-section J to HEC-2 cross-section 125 (station 9,341) using the downstream reach lengths as measured from FIS cross-section K just upstream of Kernville Road bridge (station 9,391). Cross-sections were added upstream of the bridge at the surveyed river stations from station 9,391 to station 12,095.
- Where bathymetry data was missing, the main channel elevations were updated to match the HEC-2 channel bathymetry from station 8,792 to station 9,341 and estimated by interpolation between cross-sections from station 9,391 to station 12,095.
- Roughness coefficients (Manning's "n" values) were set to match those from the DEM.
- Levees were added to the west side of cross-sections from station 8,792 to station 10,865.
- The Kernville Road bridge was added to the model with the high chord of the bridge deck linearly interpolated between the high points at either end of the bridge in the topographic survey. The low chord of the bridge deck was assumed to be five feet below the high chord as documented in the HEC-2 data. The bridge was skewed 24 degrees as measured from the topographic survey. The piers were added at the surveyed locations and widths.
- An ineffective flow area was added to cross-sections extending upstream from the Kernville Road bridge western abutment at 45 degrees, from station 9,391 to station 10,406.

A hydraulic work map showing the updated cross-sections in addition to the FEMA cross-sections, floodway, and special flood hazard areas is shown in Figure 2 of Appendix B.

3.3 Proposed Conditions Model

A Proposed Conditions Model (PCM) was developed to incorporate the proposed improvements into the existing conditions model. The PCM was created by modifying the ECM according to the following methodology:

Adding the calculated obstructed area of 23.5 square feet to the cross-section at station 9,498 at the proposed intake structure.

3.4 Model results

The DEM, ECM, and PCM were run as subcritical, steady flow with the FIS 100-year peak discharge of 69,000 cfs and a downstream boundary condition of the water surface elevation at cross-section J that is published in the FIS. The water surface elevation results from the DEM were within 0.5 feet of the published FIS water surface elevations, as shown in Table 1 and Figure 2. The water surface elevation results for the ECM vary from the results for the DEM due to the updated cross-section elevations and bridge geometry from the topographic survey.

The water surface elevation results from the PCM were compared to the ECM to determine the magnitude of the flooding impact attributed to the proposed structures. The results show a decrease of the water surface elevation at the obstructed cross-section (station 9,498) of 0.06 feet and an increase of 0.09 feet at the cross-section just upstream of the proposed improvements (station 9,606). The decrease in water surface elevation at the obstruction is expected as the velocity increases due to the reduced flow area. The increase in water surface elevation upstream of the obstruction is expected as the flow backs up due to the constricted flow area downstream. The water surface elevation comparison is summarized in Table 1 and Figure 2 shows the water surface profiles for each model. The complete hydraulic results are included Appendix C.

Table 1 Water surface elevation comparison for the 100-year peak discharge

		Water Su	rface Elevat	ion (ft, NAV	′D 88)	
Description	River Station (ft)	FIS	DEM	ECM	PCM	PCM - ECM
US Study Limit FIS cross-section M	12,095	2663.80	2663.79	2660.99	2660.99	0.00
	11,480	-	-	2659.18	2659.18	0.00
FIS cross-section L	10,865	2656.00	2656.45	2655.01	2655.01	0.00
	10,606	-	-	2654.46	2654.46	0.00
	10,406	-	-	2653.92	2653.92	0.00
	10,207	-	-	2653.92	2653.92	0.00
	10,006	-	-	2653.42	2653.42	0.00
	9,806	-	-	2650.44	2650.44	0.00
	9,606	-	-	2649.96	2650.05	0.09
Proposed Intake Structure	9,498	-	-	2647.48	2647.42	-0.06
	9,414	-	-	2648.36	2648.36	0.00
US Kernville Road bridge FIS cross-section K	9,391	2647.40	2647.27	2647.44	2647.44	0.00
DS Kernville Road bridge HEC-2 cross-section 125	9,341	-	2646.04	2646.71	2646.71	0.00
HEC-2 cross-section 1	9,326	-	2647.46	2647.09	2647.09	0.00
HEC-2 cross-section 2	9,224	-	2646.11	2646.67	2646.67	0.00
HEC-2 cross-section 3	9,122	-	2646.41	2646.69	2646.69	0.00
HEC-2 cross-section 4	9,020	-	2646.22	2645.67	2645.67	0.00
DS Study Limit FIS cross-section J	8,792	2645.20	2645.20	2645.20	2645.20	0.00

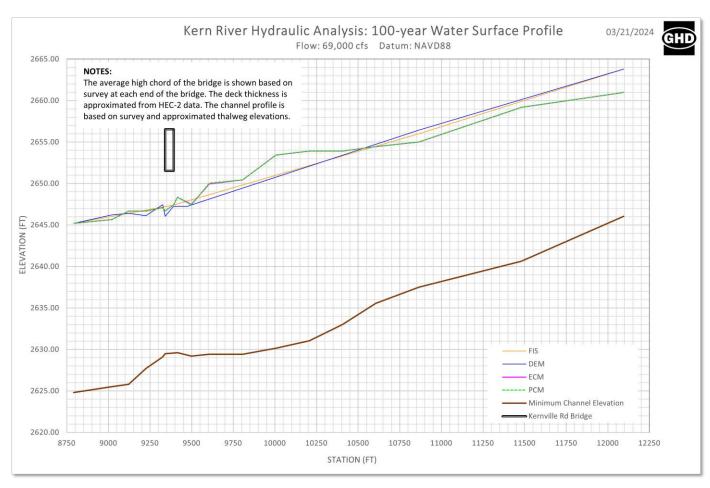


Figure 2 Water surface profile comparison for the 100-year peak discharge

4. Conclusion

This hydraulic analysis demonstrates that the proposed improvements for the Kern River Intake Replacement Project will result in a rise in the 100-year flood water surface elevation of up to 0.09 feet extending approximately 110 feet upstream of the project. This rise results in a slight expansion of the flood inundation extents in this area as compared to the ECM. Per coordination with the Central Valley Flood Protection Board, a rise of less than 0.1 feet meets their criterion for a No-Rise Certification.

5. References

FEMA. (2021). Flood Insurance Study - Kern County, California and Incorporated Areas.

Appendices

Appendix A FEMA Information

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain eros desided information in areas where Base Flood Elevations the Flood Profess and Floodway Data andre Summary of Stithuete Elevations takes contained within the Flood Instances Blooky Floor and the accompanies contained within the Flood Instances Blooky Floor and the accompanies rounded whole-boot elevations. These BFEs are intended for flood instances ming purposes of year defined not be used as the sole some of flood entire the profession of the Flood instances are some of flood instances and the profession of the Flood instances are some of flood instances and the profession of the Flood instances are some of flood instances and the profession of the Flood instances are some of flood instances and the profession instances are some of flood instances and the profession instances are some of flood instances and the profession instances are some of flood instances and the profession instances are some of floor instances and the profession instances are some of flood instances and the profession instances are some of flood instances and the profession instances are some of flood instances and the profession instances are some of flood instances and the profession instances are some of flood instances and the profession instances are some of flood instances and the profession instances are some of flood instances.

losastal Base Flood Elevations shown on this map apply only landward f O.º North American Vertical Datum of 1989 (NAVD 88). Users oil this IRIM should be aware that coastal flood elevations are also provided in the unimary of Silhwater Elevations table in the Flood elevation Subject or this jurisdiction. Elevations shown in the Summary of Silhwater Elevations the should be used for construction and/or floodshim management purposes

countaries of the **floodways** were computed at cross sections and interpolate etween cross sections. The floodways were based on hydraulic considerations thit regard to requirements of the National Flood Insurance Program. Floodway data are provided in the Flood Insurance.

Tertain areas not in Special Flood Hazard Areas may be protected by flood ontrol structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures or this intriction.

he projection used in the preparation of this map was Universal Transverse fercator (UTM) zone 11. The horizontal datum was NAOS3, GRS1998, pheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in sight positional ifferences in map features across jurisdiction boundaries. These differences on tot affect the accuracy of this FIRM.

ood elevations on this map are referenced to the North American Vertical time of 1988. These flood elevations must be compared to structure and time of 1988. These flood elevations community of the compared to structure and practical distum. For information practical distum, for the compared to the compared to the compared to the North American Vertical Datum of 1988, visit the National Gloodiect of the North American Vertical Datum of 1988, visit the National Gloodiect to the North American Vertical Datum of 1988, visit the National Gloodiect to the North American Vertical School of the North American Vertical Programma (Secondisc) when the North American Vertical Programma (National Pro

Survey at the following address:

NGS Information Services

NOAA, N/NGS12

Mattend Condition Survey

obtain current, elevation, description, and/or location information for bench mark lown on this map, please contact the Information Services Branch of stional Geodetic Survey at (301) 713–3242, or visit its website

Base map information shown on this FIRM was derived from USDA –Farm Serv Agency –Aerial Photography Field Office dated 2005 and from U.S. Geological Survey Digital Orthophoto Quadrangles produced at a scale of 1:12,000 from

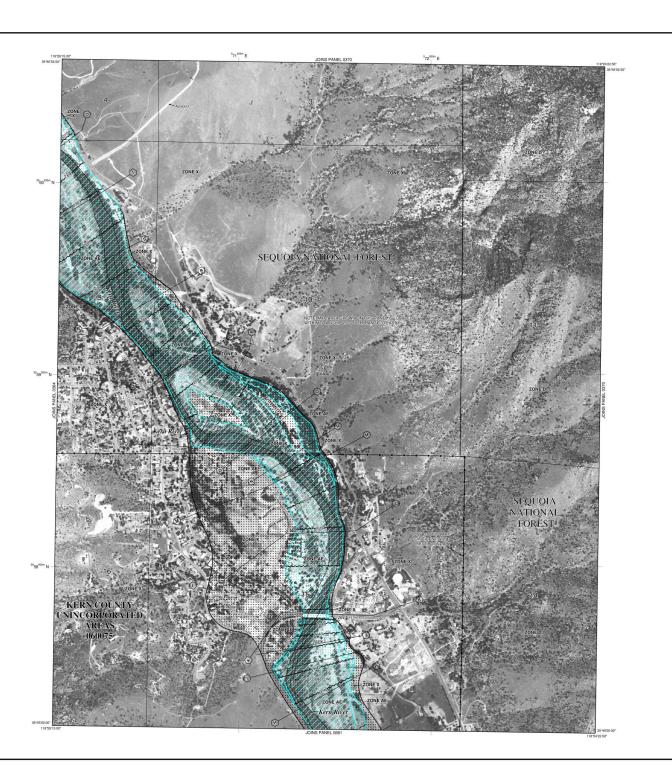
This map reflects more detailed and up-to-date stream channel configurations han those shown on the previous FIRM for this juridiction. The floodplains and floodways that were transferred from the previous FIRM may have beer quisted to conform to these new stream channel configurations. As a sesuit, the Flood Profiles and Floodway Data tables in the Flood Insurance budy report (which contains authoritative hydrauch data) may reflect stream

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact propropriate community officials to verify current comporate limit locations.

lease refer to the separately printed Map Index for an overview map of the ounty showing the layout of map panels; community map repository addresses nd a Listing of Communities table containing National Flood Insurance Program altes for each community as well as a listing of the panels on which each ommunity is located.

Contact the FEMA Map Service Center at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9500 and its webster at http://www.nsc.fema.gov/.

If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2527) or visit the FEMA website at http://www.fema.ov.



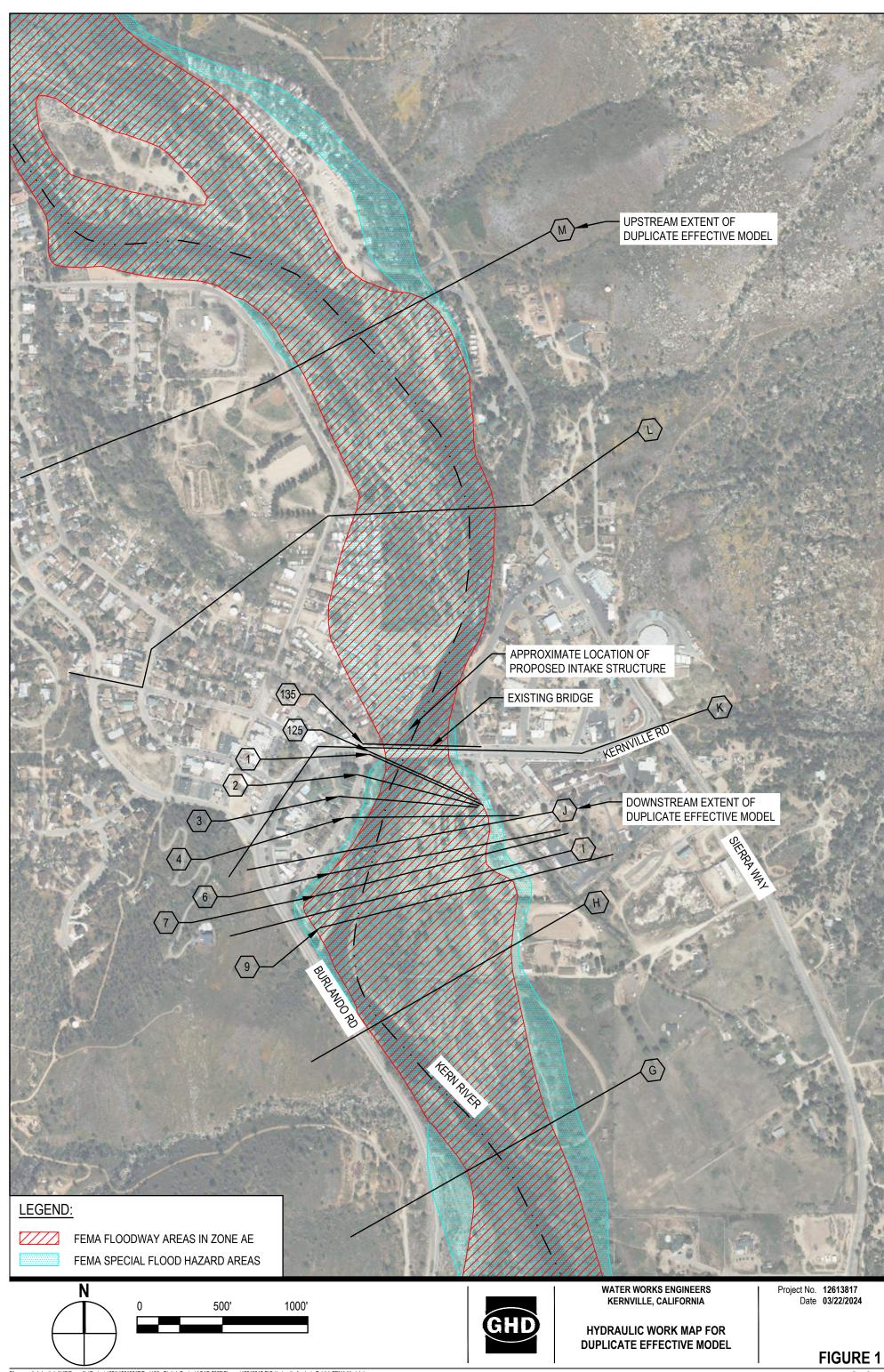
LEGEND SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Floodings determined. 70NF 499 18888 OTHER FLOOD AREAS OTHER AREAS Areas determined to be outside the 0.2% annual chance floodplain. Areas in which flood hazards are undetermined, but possible. 7///// COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS 22.52 OTHERWISE PROTECTED AREAS (OPAs) 1% annual chance floodplain boundary 0.2% annual chance floodplain boundary Floodway boundary Zone D boundary CBRS and OPA boundary Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities. Base Flood Elevation line and value; elevation in feet* (EL 987) Base Flood Elevation value where uniform within zone elevation in feet* A Cross section line Transect line 9710730", 3212230" 4275000TM 1000-meter Universal Transverse Mercator grid ticks, zone 11 S000-foot grid ticks: California State Plane coordinate system, V zone (FIPSZONE 0405), Lambert Conformal Conic 6000000 M DX5510 Bench mark (see explanation in Notes to Users section of this FIRM panel) . M1.5 River Mile MAP REPOSITORIES Refer to Map Repositories list on Map Index EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP September 26, 2008 EFFECTIVE DATE(S) OF PEVISION(S) TO THIS PANEL For community map revision history prior to countywide mapping, refer to the Communit Map History table located in the Flood Insurance Study report for this jurisdiction. To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood insurance Program at 1-800-638-6620. 250 0 SCALE 1" = 500' PANEL 0368E **FIRM** FLOOD INSURANCE RATE MAP KERN COUNTY, CALIFORNIA AND INCORPORATED AREAS PANEL 368 OF 4125 CONTAINS: COMMUNITY NUMBER PANEL SUFFIX

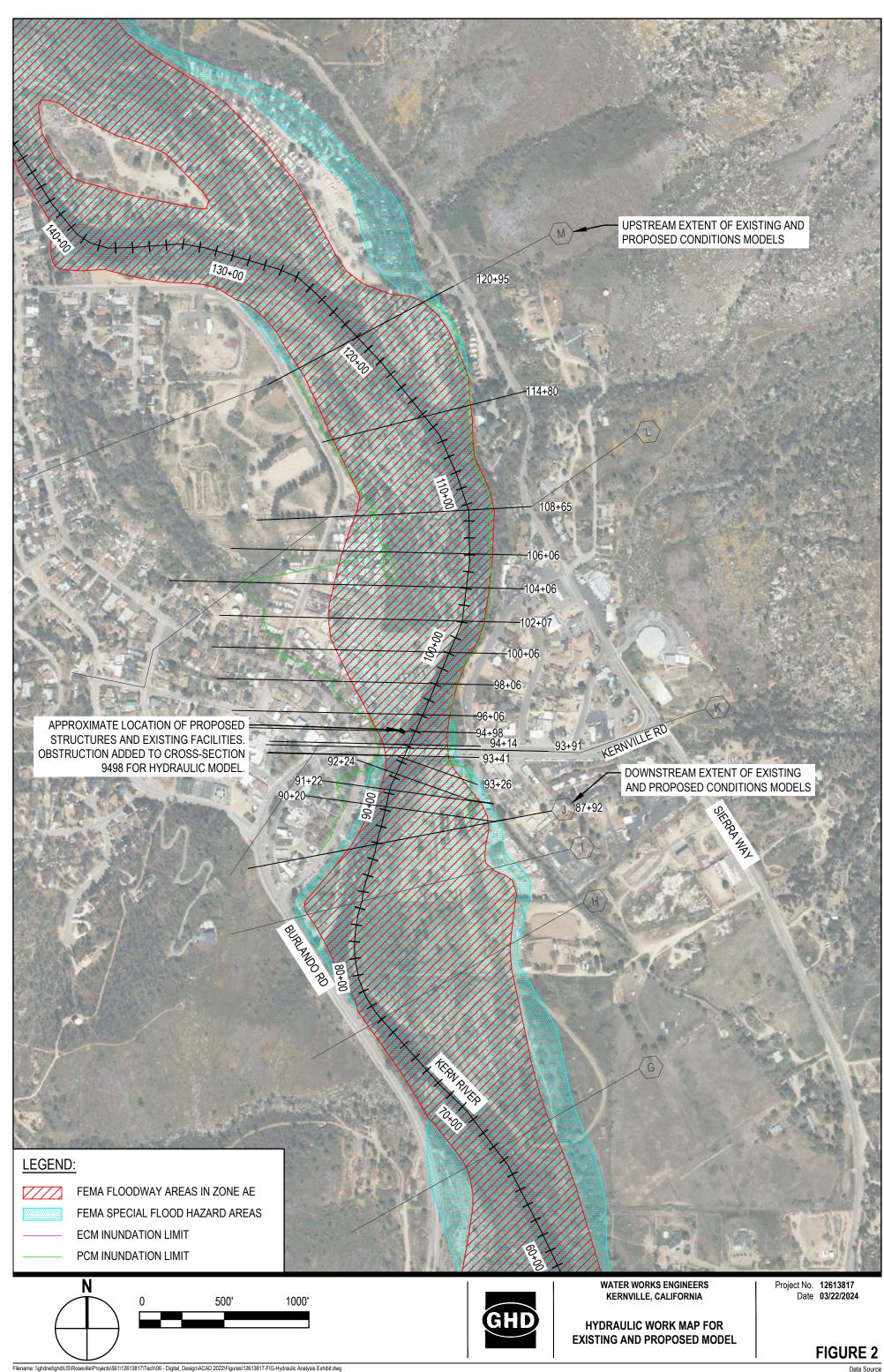
EFFECTIVE DATE

Federal Emergency Management Agency

Appendix B

Hydraulic Work Maps





Appendix C Hydraulic Results

Kern River Hydraulic Analysis

Project: Kern River Intake Replacement Project

Project #: 12613817 Updated: 3/21/2024 By: A Finn



Obstruction calculations for project at cross section 9498

Obstruction Conditions

Condition	Obstruction Area (sf)	Obstructions
Existing (ECM)	0.00	None
Proposed (PCM)	23.53	New Intake, new electrical stand

	ECM	PCM
100-yr Water Surface Elevation at XS 9498 (69,000cfs) (ft)	2647.48	2647.43

Proposed Condition

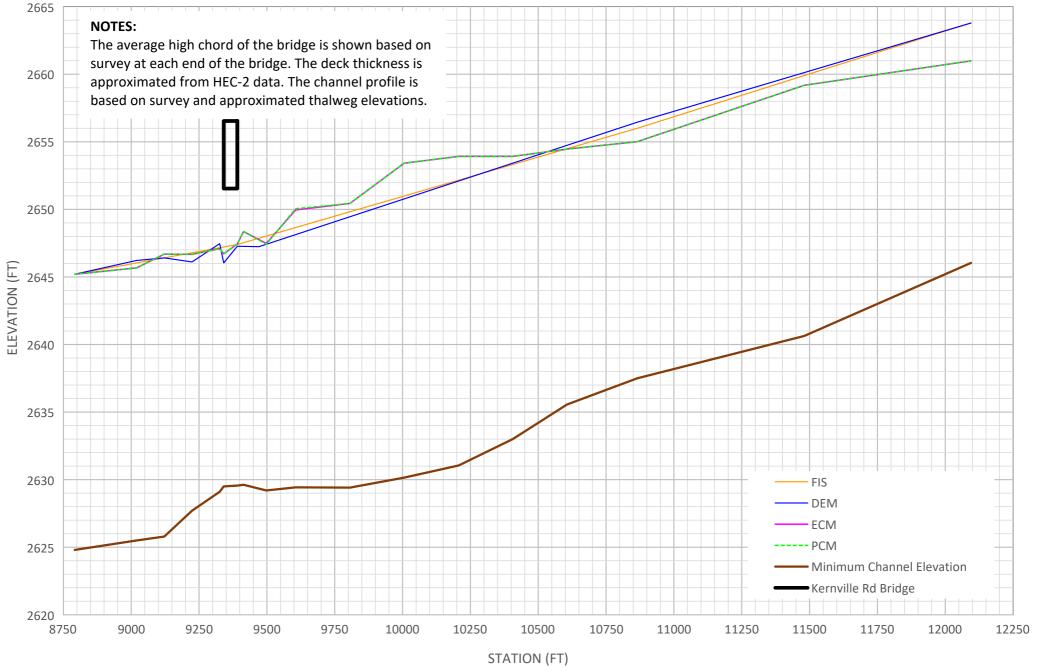
Obstruction	Top Elevation (ft)	p Elevation (ft) Bottom Wid		Obstruction Area (sf)
New Intake Structure	-	-	-	0.0
New Electrical Stand	2654.5	2642	4.3	23.5
		P	roposed Total	23.5

*installed below existing grade

Kern River Hydraulic Analysis: 100-year Water Surface Profile 03/21/2024

Flow: 69,000 cfs Datum: NAVD88

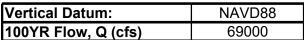


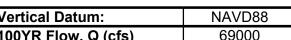


Kern River FIS Water Surface Elevation

Project: Kern River Intake Replacement Project

Project #: 12613817 **Updated:** 8/2/2023





Notes:

- FIS: FEMA Flood Insurance Study model

				Table 23 Floodway Data				
HEC-2 XS #	FIS XS	HEC-2 Station (ft)	Adjusted Station (ft)	Regulatory WS Elevation (ft)	Without Floodway WS Elevation (ft)	With Floodway WS Elevation (ft)		
150	M	12151	12095	2663.80	2663.8	2663.9		
140	L	10901	10865	2656.00	2656	2656		
130	K	9391	9391	2647.40	2647.4	2647.5		
5	J	8835	8792	2645.20	2645.2	2645.3		



Kern River HEC-RAS: DEM Output Project: Kern River Intake Replacement Project Project #: 12613817 Updated: 3/21/2024

Vertical Datum:	NAVD88
100YR Flow, Q (cfs)	69000



- DEM: Duplicate Efffective Model from HEC2

HEC-2 XS #	FIS XS	HEC-2 River Sta	Adjusted River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
		(ft)	(ft)		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
150	M	12151	12095	PF 1	69000	2644	2663.79		2665.36	0.003746	12.08	7657.69	1344.47	0.61
140	L	10901	10865	PF 1	69000	2640.5	2656.45	2655.54	2659.24	0.006746	15.24	6801.54	1927.63	0.8
135		9471	9471	PF 1	69000	2631.3	2647.24		2650.74	0.005287	16.36	5071.64	457.41	0.75
130	K	9391	9391	PF 1	69000	2630.8	2647.27	2643.42	2650.17	0.00438	14.6	5305.9	471.7	0.66
125		9341	9341	PF 1	69000	2630.8	2646.04		2649.62	0.005653	15.6	4781.52	462.81	0.74
1		9326	9326	PF 1	69000	2629.1	2647.46		2648.94	0.002096	10.01	7441.8	643.55	0.46
2		9224	9224	PF 1	69000	2627.7	2646.11		2648.56	0.003804	13.06	6050.79	666.57	0.61
3		9122	9122	PF 1	69000	2625.8	2646.41		2647.93	0.003065	10.17	7122.3	703.94	0.53
4		9020	9020	PF 1	69000	2625.5	2646.22		2647.6	0.002597	9.79	7623.87	756.56	0.49
5	J	8835	8792	PF 1	69000	2624.8	2645.20	2641.24	2647.03	0.003006	11.34	7237.91	1002.8	0.54



Kern River HEC-RAS: Existing Conditions Model Output Project: Kern River Intake Replacement Project

Project #: 12613817 **Updated:** 3/21/2024

Vertical Datum:	NAVD88
100YR Flow, Q (cfs)	69000

Notes:

- ECM: Existing Conditions Model

- Kernville Road Bridge at station 9366

- Obstruction added at XS 9498

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Reach 1	12095	PF 1	69000	2646.04	2660.99	2660.58	2663.56	0.006192	16.25	6184.1	906.07	0.79
Reach 1	11480	PF 1	69000	2640.63	2659.18		2660.83	0.002789	12.39	7535.96	842.66	0.55
Reach 1	10865	PF 1	69000	2637.5	2655.01	2654.25	2658.24	0.006021	16.38	5383.52	683.94	0.77
Reach 1	10606	PF 1	69000	2635.56	2654.46	2652.34	2656.67	0.004049	13.81	6077.71	602.36	0.63
Reach 1	10406	PF 1	69000	2633	2653.92	2651.19	2655.82	0.003557	13.56	8087.16	1490.12	0.6
Reach 1	10207	PF 1	69000	2631.05	2653.92	2650.88	2655.11	0.001825	10.78	9706.69	1310.78	0.44
Reach 1	10006	PF 1	69000	2630.15	2653.42	2650.17	2654.75	0.001645	11.29	9261.4	1152.12	0.44
Reach 1	9806	PF 1	69000	2629.41	2650.44	2650.44	2653.99	0.004683	17.03	5562.7	733.88	0.7
Reach 1	9606	PF 1	69000	2629.43	2649.96	2647.62	2652.83	0.003566	14.86	5713.79	555.14	0.63
Reach 1	9498	PF 1	69000	2629.2	2647.48	2647.26	2652.13	0.006876	18.22	4480.31	469.06	0.84
Reach 1	9414	PF 1	69000	2629.62	2648.36	2644.69	2651.16	0.003606	13.93	5494.83	441.42	0.62
Reach 1	9391	PF 1	69000	2629.56	2647.44	2645.05	2650.99	0.005518	15.37	4675.99	376.08	0.73
Reach 1	9341	PF 1	69000	2629.5	2646.71	2643.8	2650	0.00478	14.71	4858.69	379.98	0.68
Reach 1	9326	PF 1	69000	2629.1	2647.09	2643.75	2649.74	0.004023	13.48	5709.26	582.26	0.63
Reach 1	9224	PF 1	69000	2627.7	2646.67	2643.42	2649.33	0.004122	13.38	5732.32	647.58	0.63
Reach 1	9122	PF 1	69000	2625.79	2646.69	2641.72	2648.83	0.002814	12.04	6573.96	814.51	0.53
Reach 1	9020	PF 1	69000	2625.5	2645.67	2642.91	2648.41	0.004527	13.59	5692.51	741.03	0.66
Reach 1	8792	PF 1	69000	2624.8	2645.2	2642.34	2647.29	0.003657	12.65	6921.02	929.59	0.6

Kernville Road Bridge

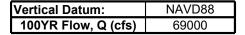
River Station (ft)	Elevation (ft)	
9391	2656.54	High Chord
9391	2651.54	Low Chord
9341	2651.54	Low Chord
9341	2656.54	High Chord
9391	2656.54	High Chord



Kern River HEC-RAS:Proposed Conditions Model Output

Project: Kern River Intake Replacement Project

Project #: 12613817 Updated: 3/21/2024



Notes:

- PCM: Proposed Conditions Model

- Kernville Road Bridge at station 9366

- Obstruction added at XS 9498

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Reach 1	12095	PF 1	69000	2646.04	2660.99	2660.58	2663.56	0.006192	16.25	6184.1	906.07	0.79
Reach 1	11480	PF 1	69000	2640.63	2659.18		2660.83	0.002789	12.39	7535.96	842.66	0.55
Reach 1	10865	PF 1	69000	2637.5	2655.01	2654.25	2658.24	0.006021	16.38	5383.52	683.94	0.77
Reach 1	10606	PF 1	69000	2635.56	2654.46	2652.34	2656.67	0.004049	13.81	6077.71	602.36	0.63
Reach 1	10406	PF 1	69000	2633	2653.92	2651.19	2655.82	0.003557	13.56	8087.16	1490.12	0.6
Reach 1	10207	PF 1	69000	2631.05	2653.92	2650.88	2655.11	0.001825	10.78	9706.69	1310.78	0.44
Reach 1	10006	PF 1	69000	2630.15	2653.42	2650.17	2654.75	0.001645	11.29	9261.13	1152.12	0.44
Reach 1	9806	PF 1	69000	2629.41	2650.44	2650.44	2653.99	0.004683	17.03	5562.7	733.88	0.7
Reach 1	9606	PF 1	69000	2629.43	2650.05	2647.62	2652.88	0.003495	14.76	5762.87	559.9	0.62
Reach 1	9498	PF 1	69000	2629.2	2647.42	2647.3	2652.17	0.007132	18.37	4431.03	468.55	0.85
Reach 1	9414	PF 1	69000	2629.62	2648.36	2644.69	2651.16	0.003606	13.93	5494.83	441.42	0.62
Reach 1	9391	PF 1	69000	2629.56	2647.44	2645.05	2650.99	0.005518	15.37	4675.99	376.08	0.73
Reach 1	9341	PF 1	69000	2629.5	2646.71	2643.8	2650	0.00478	14.71	4858.69	379.98	0.68
Reach 1	9326	PF 1	69000	2629.1	2647.09	2643.75	2649.74	0.004023	13.48	5709.26	582.26	0.63
Reach 1	9224	PF 1	69000	2627.7	2646.67	2643.42	2649.33	0.004122	13.38	5732.32	647.58	0.63
Reach 1	9122	PF 1	69000	2625.79	2646.69	2641.72	2648.83	0.002814	12.04	6573.96	814.51	0.53
Reach 1	9020	PF 1	69000	2625.5	2645.67	2642.91	2648.41	0.004527	13.59	5692.51	741.03	0.66
Reach 1	8792	PF 1	69000	2624.8	2645.2	2642.34	2647.29	0.003657	12.65	6921.02	929.59	0.6



Kern River Water Surface Elevation Comparison

Project: Kern River Intake Replacement Project

Project #: 12613817 Updated: 3/21/2024

Vertical Datum:	NAVD88
100YR Flow, Q (cfs):	69,000

Notes:

- Obstruction added at:

XS 9498

- Kernville Road Bridge at station 9366

- ECM: Existing Conditions Model

- PCM: Proposed Conditions Model

Obstruction Conditions

Condition	Obstruction Area (sf)	Obstructions		
Existing (ECM)	0.00	None		
Proposed (PCM)	23.53	New Intake, new electrical stand		

		Water Surface Elevation (ft)		Comparison (ft)
FEMA XS #	Station (ft)	ECM	PCM	PCM - ECM
FIS XS M	12095	2660.99	2660.99	0.00
	11480	2659.18	2659.18	0.00
FIS XS L	10865	2655.01	2655.01	0.00
	10606	2654.46	2654.46	0.00
	10406	2653.92	2653.92	0.00
	10207	2653.92	2653.92	0.00
	10006	2653.42	2653.42	0.00
	9806	2650.44	2650.44	0.00
	9606	2649.96	2650.05	0.09
	9498	2647.48	2647.42	-0.06
	9414	2648.36	2648.36	0.00
FIS XS K	9391	2647.44	2647.44	0.00
HEC-2 XS 125	9341	2646.71	2646.71	0.00
HEC-2 XS 1	9326	2647.09	2647.09	0.00
HEC-2 XS 2	9224	2646.67	2646.67	0.00
HEC-2 XS 3	9122	2646.69	2646.69	0.00
HEC-2 XS 4	9020	2645.67	2645.67	0.00
HEC-2 XS 5 (FIS J)	8792	2645.20	2645.20	0.00

*obstruction location



HEC-RAS HEC-RAS 6.3.1 September 2022 U.S. Army Corps of Engineers Hydrologic Engineering Center 609 Second Street Davis, California

Χ	Χ	XXXXXX	XX	XX		XX	XX	>	(X	XXXX
Χ	Χ	Χ	Χ	Χ		Χ	Χ	Χ	Χ	Χ
Χ	Χ	Χ	Χ			Χ	Χ	Χ	Χ	Χ
XXX	XXXX	XXXX	Χ		XXX	XX	XX	XXX	ΧXX	XXXX
Χ	Χ	Χ	Χ			Χ	Χ	Χ	Χ	Χ
Χ	Χ	Χ	Χ	Χ		Χ	Χ	Χ	Χ	Χ
Χ	Χ	XXXXXX	XX	XX		Χ	Χ	Χ	Χ	XXXXX

PROJECT DATA

Project Title: Kern River Project File : KernRiver.prj

Run Date and Time: 3/21/2024 3:14:24 PM

Project in English units

Project Description:

Analysis of proposed intake structure improvements at Kern River upstream of Kernville Road bridge.

PLAN DATA

Plan Title: DEM - Q100

Plan File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River HEC-RAS

Model\KernRiver.p11

Geometry Title: DEM

Geometry File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River

HEC-RAS Model\KernRiver.g11

Flow Title : DEM - Q100

Flow File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River

HEC-RAS Model\KernRiver.f03

Plan Summary Information:

Number of: Cross Sections = 10 Multiple Openings = 0

Culverts = 0 Inline Structures = 0

Bridges = 1 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.01
Critical depth calculation tolerance = 0.01
Maximum number of iterations = 20
Maximum difference tolerance = 0.3
Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary

Conveyance Calculation Method: At breaks in n values only

Friction Slope Method: Average Conveyance Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: DEM - Q100

Flow File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River HEC-RAS

Model\KernRiver.f03

Flow Data (cfs)

River Reach RS PF 1 KernRiver 1 12151 69000

Boundary Conditions

River Reach Profile Upstream

Downstream

KernRiver 1 PF 1

Known WS = 2645.2

SUMMARY OF MANNING'S N VALUES

River:KernRiver

Reach River Sta. n1 n2 n3

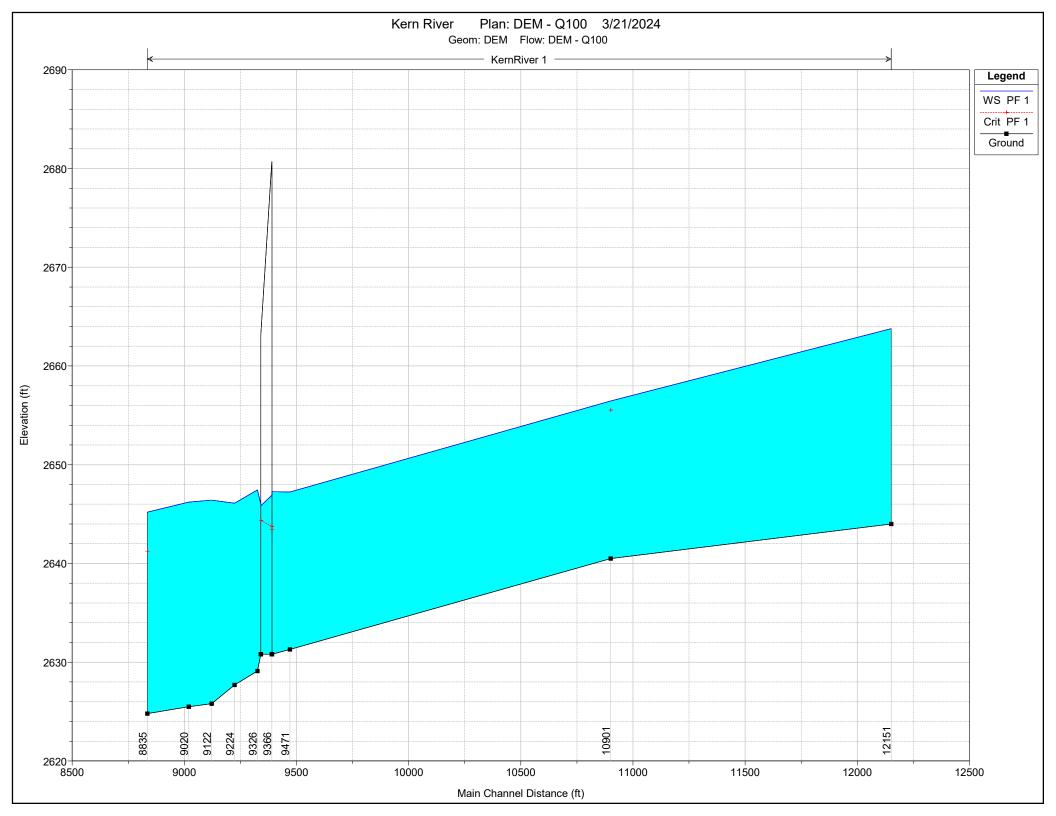
1	12151	.045	.04	.04
1	10901	.045	.04	.04
1	9471	.045	.04	.055
1	9391	.046	.041	.056
1	9366	Bridge		
1	9341	.046	.041	.043
1	9326	.046	.041	.043
1	9224	.046	.041	.043
1	9122	.046	.041	.043
1	9020	.046	.041	.043
1	8835	.046	.041	.043

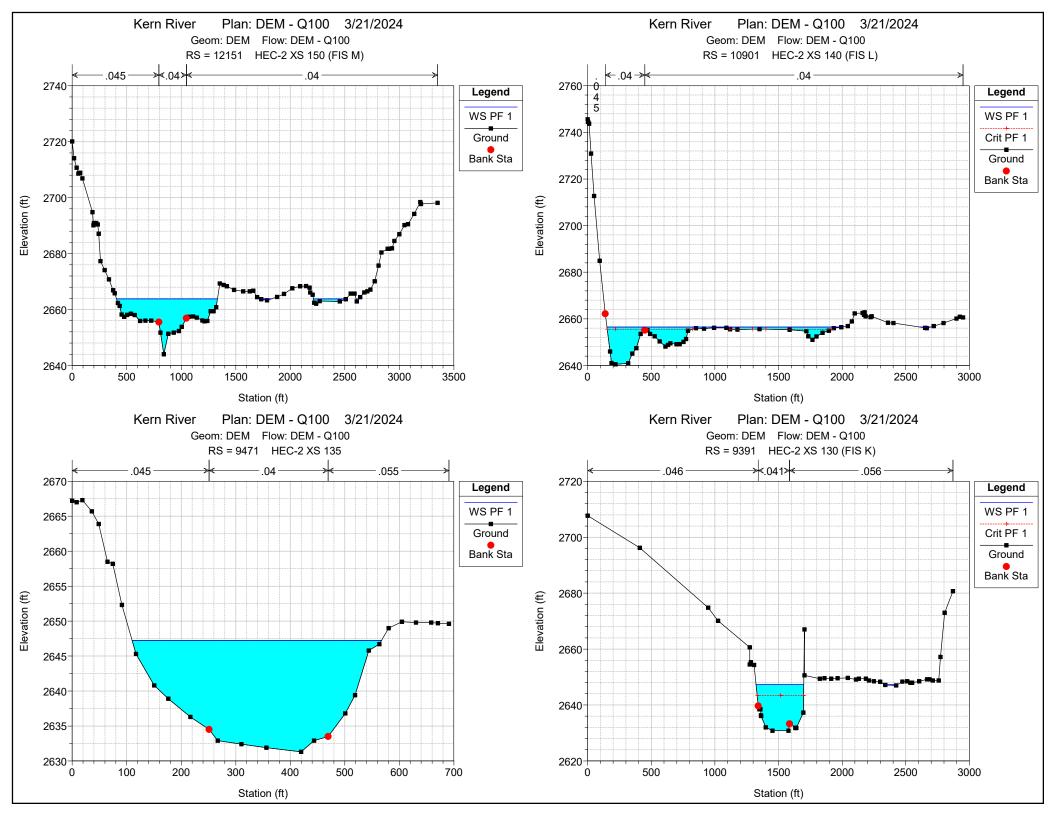
Profile Output Table - Standard Table 1

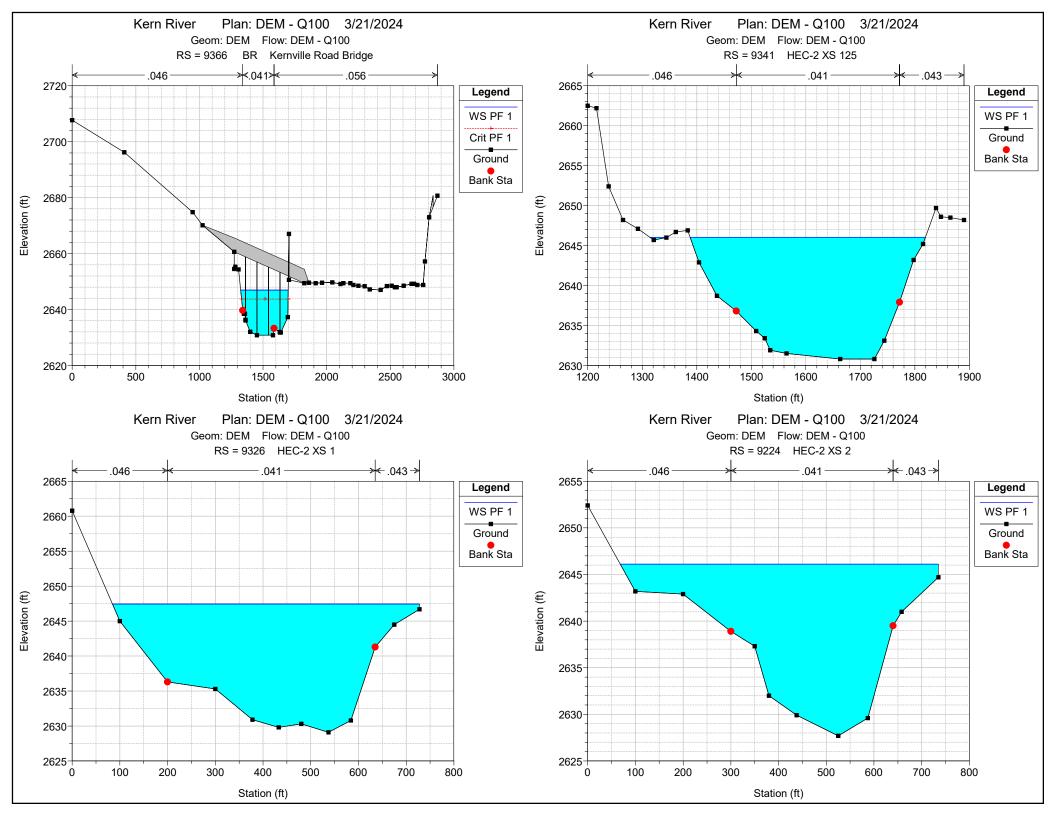
					W.S. Elev	
E.G. Elev	E.G. Slope				Froude # Chl	
			(cfs)	(ft)	(ft)	(ft)
(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)		
1	12151	PF 1	69000.00	2644.00	2663.79	
	0.003746		7657.69		0.61	
1		PF 1			2656.45	
	0.006746	15.24			0.80	
1					2647.24	
2650.74	0.005287	16.36			0.75	
1	9391	PF 1			2647.27	
2650.17	0.004380				0.66	
1	9366		Bridge			
1	9341	PF 1	69000.00	2630.80	2646.04	
_	0.005653				0.74	
1	9326				2647.46	
2648.94					0.46	
1			69000.00		2646.11	
2648.56	0.003804				0.61	
1	9122		69000.00			
2647.93	0.003065				0.53	
1	9020				2646.22	
2647.60					0.49	
1					2645.20	
2647.03	0.003006		7237.91		0.54	

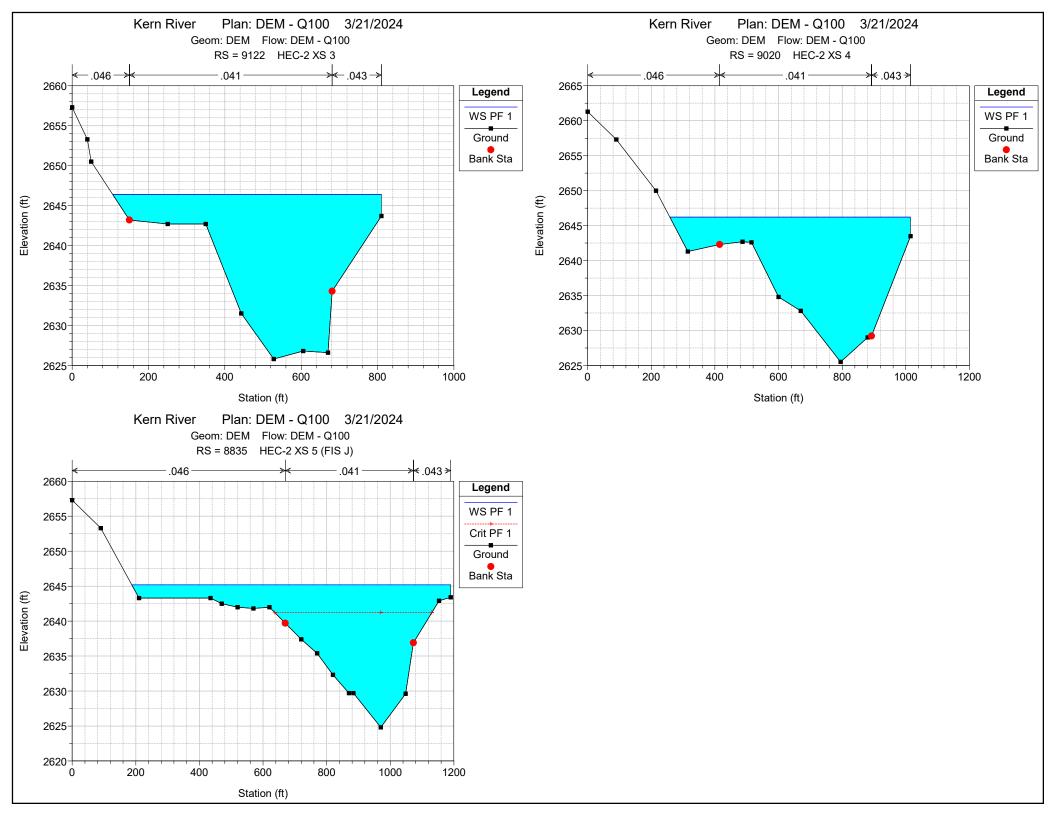
HEC-RAS Plan: DEM - Q100 River: KernRiver Reach: 1 Profile: PF 1

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
1	12151	PF 1	69000.00	2644.00	2663.79		2665.36	0.003746	12.08	7657.69	1344.47	0.61
1	10901	PF 1	69000.00	2640.50	2656.45	2655.54	2659.24	0.006746	15.24	6801.54	1927.63	0.80
1	9471	PF 1	69000.00	2631.30	2647.24		2650.74	0.005287	16.36	5071.64	457.41	0.75
1	9391	PF 1	69000.00	2630.80	2647.27	2643.42	2650.17	0.004380	14.60	5305.90	471.70	0.66
1	9366		Bridge									
1	9341	PF 1	69000.00	2630.80	2646.04		2649.62	0.005653	15.60	4781.52	462.81	0.74
1	9326	PF 1	69000.00	2629.10	2647.46		2648.94	0.002096	10.01	7441.80	643.55	0.46
1	9224	PF 1	69000.00	2627.70	2646.11		2648.56	0.003804	13.06	6050.79	666.57	0.61
1	9122	PF 1	69000.00	2625.80	2646.41		2647.93	0.003065	10.17	7122.30	703.94	0.53
1	9020	PF 1	69000.00	2625.50	2646.22		2647.60	0.002597	9.79	7623.87	756.56	0.49
1	8835	PF 1	69000.00	2624.80	2645.20	2641.24	2647.03	0.003006	11.34	7237.91	1002.80	0.54









HEC-RAS HEC-RAS 6.3.1 September 2022 U.S. Army Corps of Engineers Hydrologic Engineering Center 609 Second Street Davis, California

Χ	Χ	XXXXXX	XX	XX		XX	XX	X	X	XXXX
Χ	Χ	Χ	Χ	Χ		Χ	Χ	Χ	Χ	Χ
Χ	Χ	Χ	Χ			Χ	Χ	Χ	Χ	Χ
XXX	XXXX	XXXX	Χ		XXX	XX	XX	XXX	XXX	XXXX
Χ	Χ	Χ	Χ			Χ	Χ	Χ	Χ	Χ
Χ	Χ	Χ	Χ	Χ		Χ	Χ	Χ	Χ	Χ
Χ	Х	XXXXXX	XX	XX		Χ	Х	Χ	Х	XXXXX

PROJECT DATA

Project Title: Kern River Project File : KernRiver.prj

Run Date and Time: 3/21/2024 3:24:04 PM

Project in English units

Project Description:

Analysis of proposed intake structure improvements at Kern River upstream of Kernville Road bridge.

PLAN DATA

Plan Title: ECM - Q100

Plan File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River HEC-RAS

Model\KernRiver.p09

Geometry Title: ECM

Geometry File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River

HEC-RAS Model\KernRiver.g10

Flow Title : Q100

Flow File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River

HEC-RAS Model\KernRiver.f02

Plan Summary Information:

Number of: Cross Sections = 18 Multiple Openings = 0

Culverts = 0 Inline Structures = 0

Bridges = 1 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.01
Critical depth calculation tolerance = 0.01
Maximum number of iterations = 20
Maximum difference tolerance = 0.3
Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary

Conveyance Calculation Method: At breaks in n values only

Friction Slope Method: Average Conveyance Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: Q100

Flow File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River HEC-RAS

Model\KernRiver.f02

Flow Data (cfs)

River Reach RS PF 1 Kern River Reach 1 12095 69000

Boundary Conditions

River Reach Profile Upstream

Downstream

Kern River Reach 1 PF 1

Known WS = 2645.2

SUMMARY OF MANNING'S N VALUES

River:Kern River

Reach River Sta. n1 n2 n3

Reach 1	12095	.045	.04	.04
Reach 1	11480	.045	.04	.04
Reach 1	10865	.045	.04	.04
Reach 1	10606	.045	.04	.04
Reach 1	10406	.045	.04	.04
Reach 1	10207	.045	.04	.04
Reach 1	10006	.045	.04	.04
Reach 1	9806	.045	.04	.04
Reach 1	9606	.045	.04	.04
Reach 1	9498	.045	.04	.055
Reach 1	9414	.045	.04	.055
Reach 1	9391	.046	.041	.056
Reach 1	9366	Bridge		
Reach 1	9341	.046	.041	.043
Reach 1	9326	.046	.041	.043
Reach 1	9224	.046	.041	.043
Reach 1	9122	.046	.041	.043
Reach 1	9020	.046	.041	.043
Reach 1	8792	.046	.041	.043

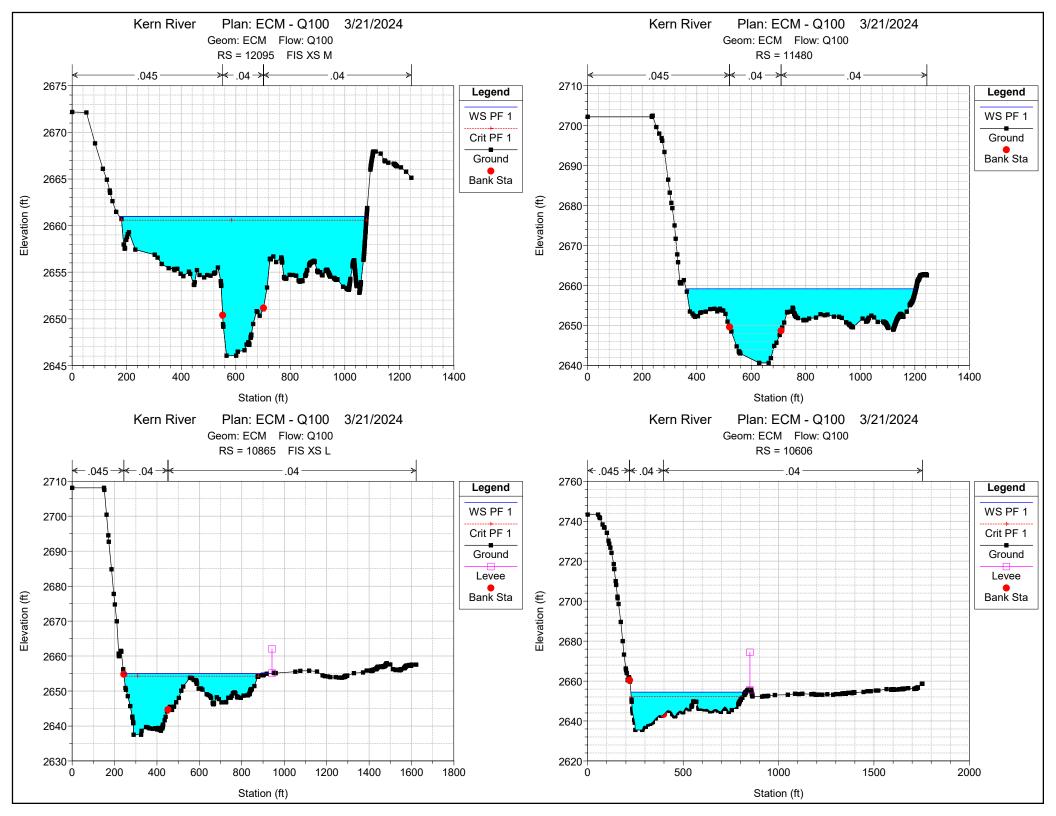
Profile Output Table - Standard Table 1

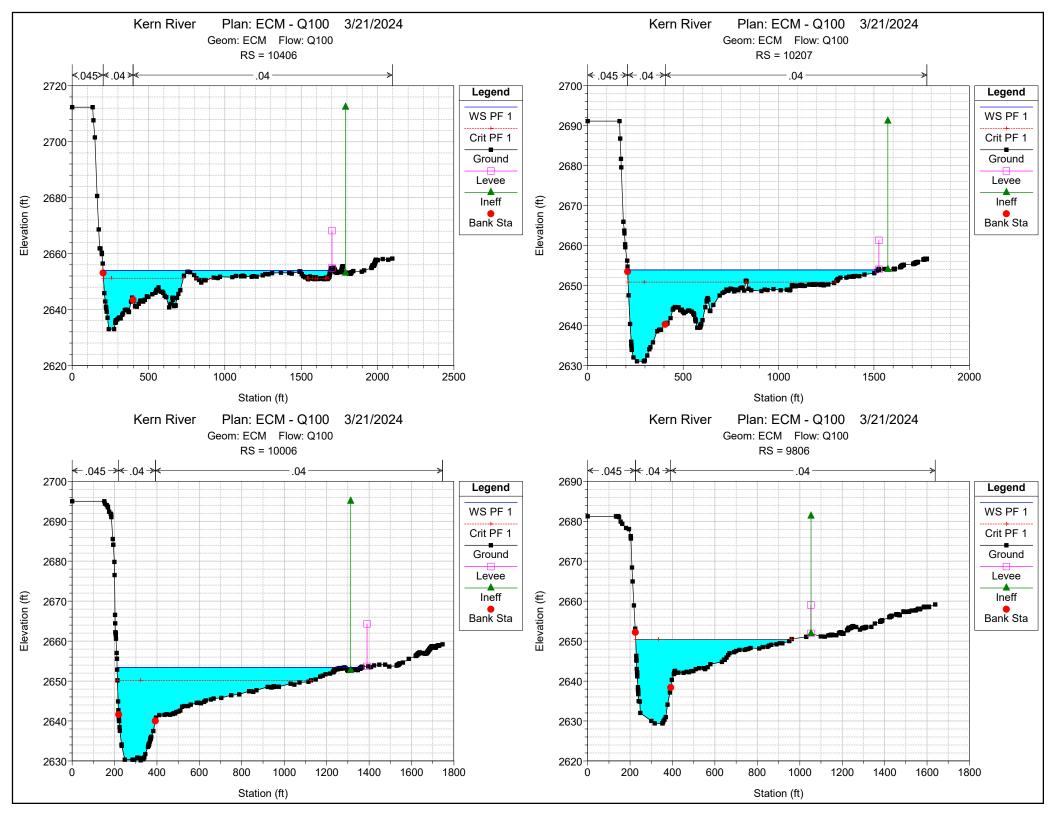
			-		W.S. Elev	Crit W.S.
E.G. Elev	E.G. Slope	Vel Chnl		•	Froude # Chl	
					(ft)	(ft)
(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)		
Reach 1	12095	PF 1	69000.00	2646.04	2660.99	2660.58
2663.56	0.006192	16.25	6184.10	906.07	0.79	
Reach 1	11480	PF 1	69000.00	2640.63	2659.18	
2660.83	0.002789	12.39	7535.96	842.66	0.55	
Reach 1	10865	PF 1	69000.00	2637.50	2655.01	2654.25
2658.24	0.006021	16.38	5383.52	683.94	0.77	
Reach 1	10606	PF 1	69000.00	2635.56	2654.46	2652.34
2656.67	0.004049	13.81	6077.71	602.36	0.63	
Reach 1	10406	PF 1	69000.00	2633.00	2653.92	2651.19
2655.82	0.003557	13.56	8087.16	1490.12	0.60	
Reach 1	10207	PF 1	69000.00	2631.05	2653.92	2650.88
2655.11	0.001825	10.78	9706.69	1310.78	0.44	
Reach 1	10006	PF 1	69000.00	2630.15	2653.42	2650.17
2654.75	0.001645	11.29	9261.40	1152.12	0.44	
Reach 1	9806	PF 1	69000.00	2629.41	2650.44	2650.44
2653.99	0.004683	17.03	5562.70	733.88	0.70	
Reach 1	9606	PF 1	69000.00	2629.43	2649.96	2647.62
2652.83	0.003566	14.86	5713.79	555.14	0.63	
Reach 1	9498	PF 1	69000.00	2629.20	2647.48	2647.26
2652.13	0.006876	18.22	4480.31	469.06	0.84	

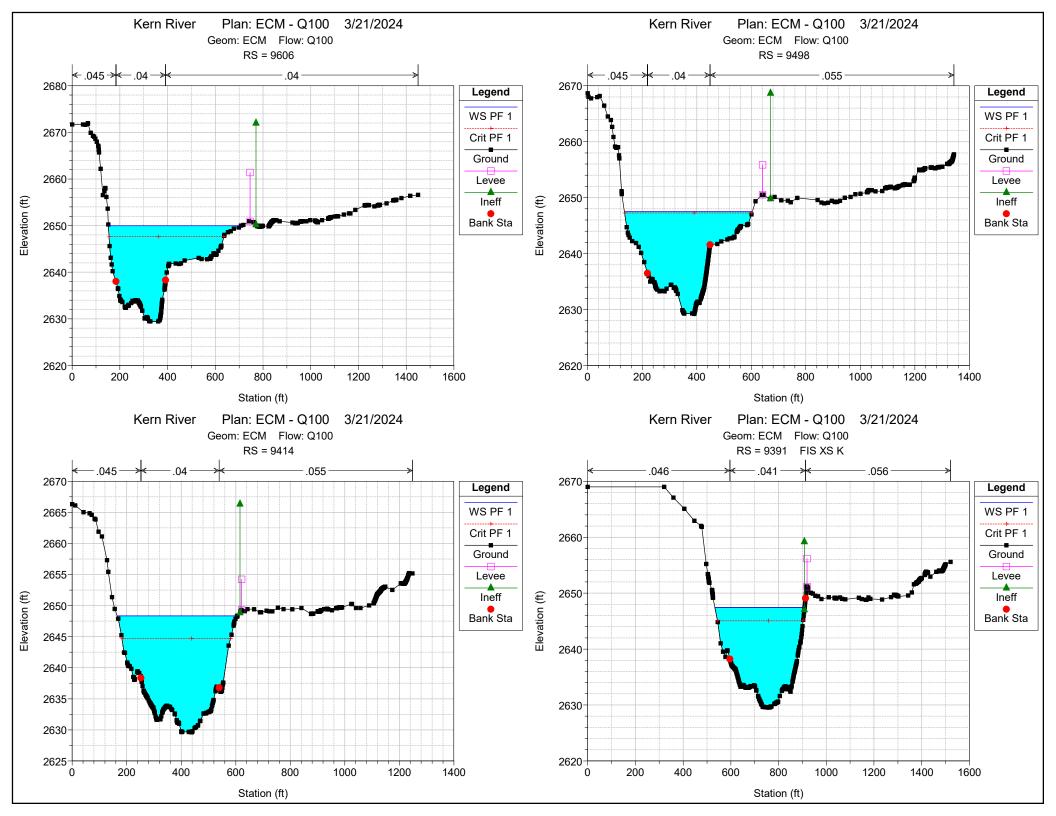
Reach 1	9414	PF 1	69000.00	2629.62	2648.36	2644.69
2651.16	0.003606	13.93	5494.83	441.42	0.62	
Reach 1	9391	PF 1	69000.00	2629.56	2647.44	2645.05
2650.99	0.005518	15.37	4675.99	376.08	0.73	
Reach 1	9366		Bridge			
Reach 1	9341	PF 1	69000.00	2629.50	2646.71	2643.80
2650.00	0.004780	14.71	4858.69	379.98	0.68	
Reach 1	9326	PF 1	69000.00	2629.10	2647.09	2643.75
2649.74	0.004023	13.48	5709.26	582.26	0.63	
Reach 1	9224	PF 1	69000.00	2627.70	2646.67	2643.42
2649.33	0.004122	13.38	5732.32	647.58	0.63	
Reach 1	9122	PF 1	69000.00	2625.79	2646.69	2641.72
2648.83	0.002814	12.04	6573.96	814.51	0.53	
Reach 1	9020	PF 1	69000.00	2625.50	2645.67	2642.91
2648.41	0.004527	13.59	5692.51	741.03	0.66	
Reach 1	8792	PF 1	69000.00	2624.80	2645.20	2642.34
2647.29	0.003657	12.65	6921.02	929.59	0.60	

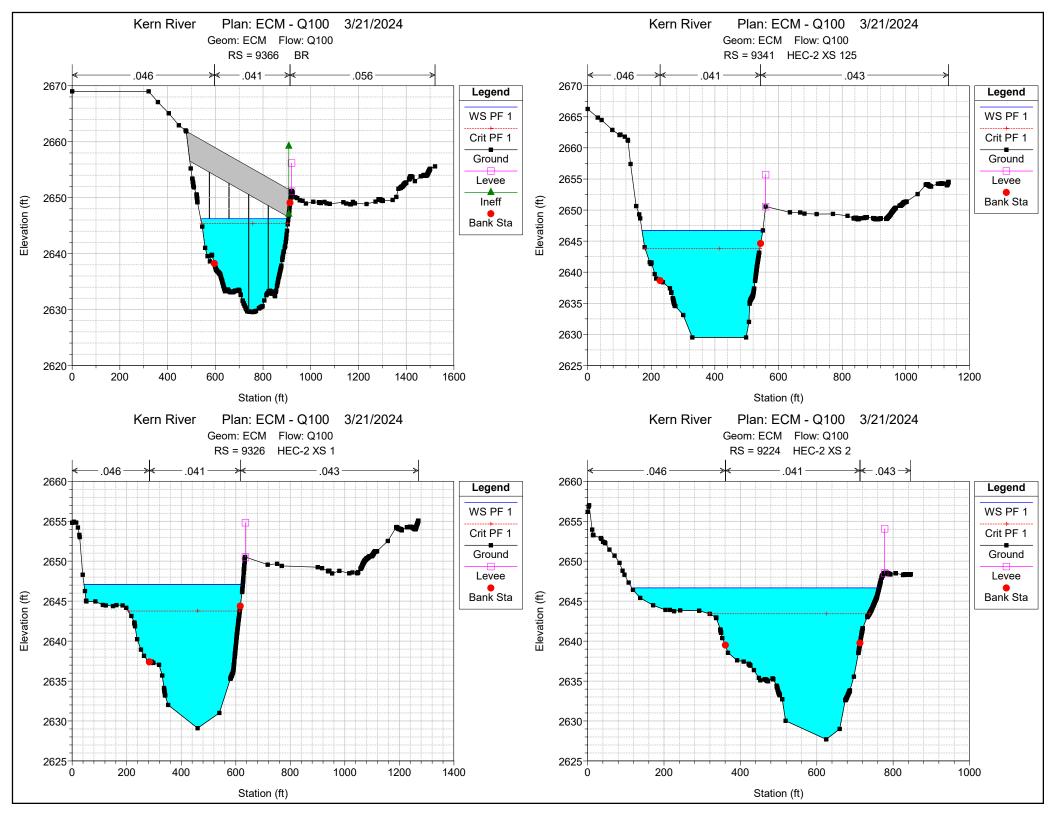
HEC-RAS Plan: ECM - Q100 River: Kern River Reach: Reach 1 Profile: PF 1

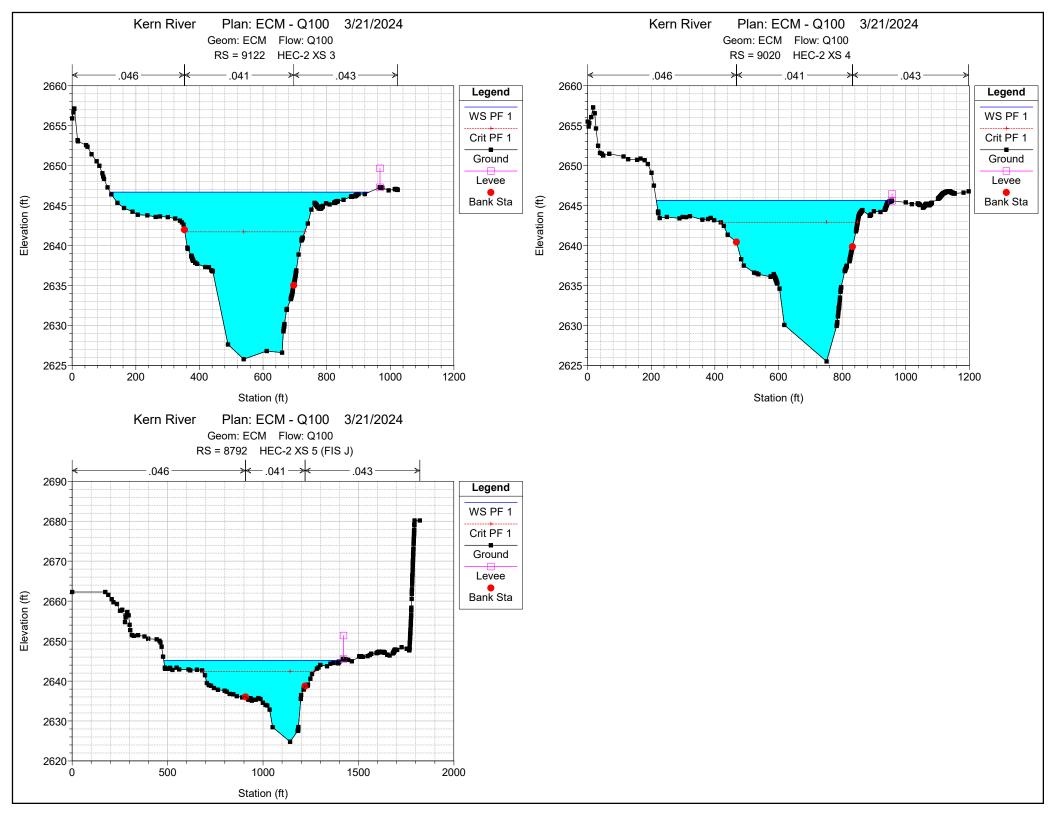
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Reach 1	12095	PF 1	69000.00	2646.04	2660.99	2660.58	2663.56	0.006192	16.25	6184.10	906.07	0.79
Reach 1	11480	PF 1	69000.00	2640.63	2659.18		2660.83	0.002789	12.39	7535.96	842.66	0.55
Reach 1	10865	PF 1	69000.00	2637.50	2655.01	2654.25	2658.24	0.006021	16.38	5383.52	683.94	0.77
Reach 1	10606	PF 1	69000.00	2635.56	2654.46	2652.34	2656.67	0.004049	13.81	6077.71	602.36	0.63
Reach 1	10406	PF 1	69000.00	2633.00	2653.92	2651.19	2655.82	0.003557	13.56	8087.16	1490.12	0.60
Reach 1	10207	PF 1	69000.00	2631.05	2653.92	2650.88	2655.11	0.001825	10.78	9706.69	1310.78	0.44
Reach 1	10006	PF 1	69000.00	2630.15	2653.42	2650.17	2654.75	0.001645	11.29	9261.40	1152.12	0.44
Reach 1	9806	PF 1	69000.00	2629.41	2650.44	2650.44	2653.99	0.004683	17.03	5562.70	733.88	0.70
Reach 1	9606	PF 1	69000.00	2629.43	2649.96	2647.62	2652.83	0.003566	14.86	5713.79	555.14	0.63
Reach 1	9498	PF 1	69000.00	2629.20	2647.48	2647.26	2652.13	0.006876	18.22	4480.31	469.06	0.84
Reach 1	9414	PF 1	69000.00	2629.62	2648.36	2644.69	2651.16	0.003606	13.93	5494.83	441.42	0.62
Reach 1	9391	PF 1	69000.00	2629.56	2647.44	2645.05	2650.99	0.005518	15.37	4675.99	376.08	0.73
Reach 1	9366		Bridge									
Reach 1	9341	PF 1	69000.00	2629.50	2646.71	2643.80	2650.00	0.004780	14.71	4858.69	379.98	0.68
Reach 1	9326	PF 1	69000.00	2629.10	2647.09	2643.75	2649.74	0.004023	13.48	5709.26	582.26	0.63
Reach 1	9224	PF 1	69000.00	2627.70	2646.67	2643.42	2649.33	0.004122	13.38	5732.32	647.58	0.63
Reach 1	9122	PF 1	69000.00	2625.79	2646.69	2641.72	2648.83	0.002814	12.04	6573.96	814.51	0.53
Reach 1	9020	PF 1	69000.00	2625.50	2645.67	2642.91	2648.41	0.004527	13.59	5692.51	741.03	0.66
Reach 1	8792	PF 1	69000.00	2624.80	2645.20	2642.34	2647.29	0.003657	12.65	6921.02	929.59	0.60











HEC-RAS HEC-RAS 6.3.1 September 2022 U.S. Army Corps of Engineers Hydrologic Engineering Center 609 Second Street Davis, California

Χ	Χ	XXXXXX	XX	XX		XX	XX	Χ	X	XXXX
Χ	Χ	Χ	Χ	Χ		Χ	Χ	Χ	Χ	Χ
Χ	Χ	Χ	Χ			Χ	Χ	Χ	Χ	Χ
XXX	XXXX	XXXX	Χ		XXX	XX	XX	XXX	XXX	XXXX
Χ	Χ	Χ	Χ			Χ	Χ	Χ	Χ	Х
Χ	Χ	Χ	Χ	Χ		Χ	Χ	Χ	Χ	Χ
Χ	Х	XXXXXX	XX	XX		Χ	Х	Х	Х	XXXXX

PROJECT DATA

Project Title: Kern River Project File : KernRiver.prj

Run Date and Time: 3/21/2024 3:14:26 PM

Project in English units

Project Description:

Analysis of proposed intake structure improvements at Kern River upstream of Kernville Road bridge.

PLAN DATA

Plan Title: PCM - Q100

Plan File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River HEC-RAS

Model\KernRiver.p01

Geometry Title: PCM

Geometry File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River

HEC-RAS Model\KernRiver.g01

Flow Title : Q100

Flow File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River

HEC-RAS Model\KernRiver.f02

Plan Summary Information:

Number of: Cross Sections = 18 Multiple Openings = 0

Culverts = 0 Inline Structures = 0

Bridges = 1 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.01
Critical depth calculation tolerance = 0.01
Maximum number of iterations = 20
Maximum difference tolerance = 0.3
Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary

Conveyance Calculation Method: At breaks in n values only

Friction Slope Method: Average Conveyance Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: Q100

Flow File : C:\Users\afinn2\OneDrive - GHD\Desktop\Models\Kern River HEC-RAS

Model\KernRiver.f02

Flow Data (cfs)

River Reach RS PF 1 Kern River Reach 1 12095 69000

Boundary Conditions

River Reach Profile Upstream

Downstream

Kern River Reach 1 PF 1

Known WS = 2645.2

SUMMARY OF MANNING'S N VALUES

River:Kern River

Reach River Sta. n1 n2 n3

Reach 1	12095	.045	.04	.04
Reach 1	11480	.045	.04	.04
Reach 1	10865	.045	.04	.04
Reach 1	10606	.045	.04	.04
Reach 1	10406	.045	.04	.04
Reach 1	10207	.045	.04	.04
Reach 1	10006	.045	.04	.04
Reach 1	9806	.045	.04	.04
Reach 1	9606	.045	.04	.04
Reach 1	9498	.045	.04	.055
Reach 1	9414	.045	.04	.055
Reach 1	9391	.046	.041	.056
Reach 1	9366	Bridge		
Reach 1	9341	.046	.041	.043
Reach 1	9326	.046	.041	.043
Reach 1	9224	.046	.041	.043
Reach 1	9122	.046	.041	.043
Reach 1	9020	.046	.041	.043
Reach 1	8792	.046	.041	.043

Profile Output Table - Standard Table 1

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.
E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl	
			(cfs)	(ft)	(ft)	(ft)
(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)		
Reach 1	12095	PF 1	69000.00	2646.04	2660.99	2660.58
2663.56	0.006192	16.25	6184.10	906.07	0.79	2000.50
Reach 1	11480	PF 1	69000.00	2640.63	2659.18	
2660.83	0.002789	12.39	7535.96	842.66	0.55	
Reach 1	10865	PF 1	69000.00	2637.50	2655.01	
2658.24	0.006021	16.38	5383.52	683.94	0.77	
Reach 1	10606	PF 1	69000.00	2635.56	2654.46	2652.34
2656.67	0.004049	13.81	6077.71	602.36	0.63	
Reach 1	10406	PF 1	69000.00	2633.00	2653.92	2651.19
2655.82	0.003557	13.56	8087.16	1490.12	0.60	
Reach 1	10207	PF 1	69000.00	2631.05	2653.92	2650.88
2655.11	0.001825	10.78	9706.69	1310.78	0.44	
Reach 1	10006	PF 1	69000.00	2630.15	2653.42	2650.17
2654.75	0.001645	11.29	9261.13	1152.12	0.44	
Reach 1	9806	PF 1	69000.00	2629.41	2650.44	2650.44
2653.99	0.004683	17.03	5562.70	733.88	0.70	
Reach 1	9606	PF 1	69000.00	2629.43	2650.05	2647.62
2652.88	0.003495	14.76	5762.87	559.90	0.62	
Reach 1	9498	PF 1	69000.00	2629.20	2647.42	2647.30
2652.17	0.007132	18.37	4431.03	468.55	0.85	

Reach 1	9414	PF 1	69000.00	2629.62	2648.36	2644.69
2651.16	0.003606	13.93	5494.83	441.42	0.62	
Reach 1	9391	PF 1	69000.00	2629.56	2647.44	2645.05
2650.99	0.005518	15.37	4675.99	376.08	0.73	
Reach 1	9366		Bridge			
Reach 1	9341	PF 1	69000.00	2629.50	2646.71	2643.80
2650.00	0.004780	14.71	4858.69	379.98	0.68	
Reach 1	9326	PF 1	69000.00	2629.10	2647.09	2643.75
2649.74	0.004023	13.48	5709.26	582.26	0.63	
Reach 1	9224	PF 1	69000.00	2627.70	2646.67	2643.42
2649.33	0.004122	13.38	5732.32	647.58	0.63	
Reach 1	9122	PF 1	69000.00	2625.79	2646.69	2641.72
2648.83	0.002814	12.04	6573.96	814.51	0.53	
Reach 1	9020	PF 1	69000.00	2625.50	2645.67	2642.91
2648.41	0.004527	13.59	5692.51	741.03	0.66	
Reach 1	8792	PF 1	69000.00	2624.80	2645.20	2642.34
2647.29	0.003657	12.65	6921.02	929.59	0.60	

HEC-RAS Plan: PCM - Q100 River: Kern River Reach: Reach 1 Profile: PF 1

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Reach 1	12095	PF 1	69000.00	2646.04	2660.99	2660.58	2663.56	0.006192	16.25	6184.10	906.07	0.79
Reach 1	11480	PF 1	69000.00	2640.63	2659.18		2660.83	0.002789	12.39	7535.96	842.66	0.55
Reach 1	10865	PF 1	69000.00	2637.50	2655.01	2654.25	2658.24	0.006021	16.38	5383.52	683.94	0.77
Reach 1	10606	PF 1	69000.00	2635.56	2654.46	2652.34	2656.67	0.004049	13.81	6077.71	602.36	0.63
Reach 1	10406	PF 1	69000.00	2633.00	2653.92	2651.19	2655.82	0.003557	13.56	8087.16	1490.12	0.60
Reach 1	10207	PF 1	69000.00	2631.05	2653.92	2650.88	2655.11	0.001825	10.78	9706.69	1310.78	0.44
Reach 1	10006	PF 1	69000.00	2630.15	2653.42	2650.17	2654.75	0.001645	11.29	9261.13	1152.12	0.44
Reach 1	9806	PF 1	69000.00	2629.41	2650.44	2650.44	2653.99	0.004683	17.03	5562.70	733.88	0.70
Reach 1	9606	PF 1	69000.00	2629.43	2650.05	2647.62	2652.88	0.003495	14.76	5762.87	559.90	0.62
Reach 1	9498	PF 1	69000.00	2629.20	2647.42	2647.30	2652.17	0.007132	18.37	4431.03	468.55	0.85
Reach 1	9414	PF 1	69000.00	2629.62	2648.36	2644.69	2651.16	0.003606	13.93	5494.83	441.42	0.62
Reach 1	9391	PF 1	69000.00	2629.56	2647.44	2645.05	2650.99	0.005518	15.37	4675.99	376.08	0.73
Reach 1	9366		Bridge									
Reach 1	9341	PF 1	69000.00	2629.50	2646.71	2643.80	2650.00	0.004780	14.71	4858.69	379.98	0.68
Reach 1	9326	PF 1	69000.00	2629.10	2647.09	2643.75	2649.74	0.004023	13.48	5709.26	582.26	0.63
Reach 1	9224	PF 1	69000.00	2627.70	2646.67	2643.42	2649.33	0.004122	13.38	5732.32	647.58	0.63
Reach 1	9122	PF 1	69000.00	2625.79	2646.69	2641.72	2648.83	0.002814	12.04	6573.96	814.51	0.53
Reach 1	9020	PF 1	69000.00	2625.50	2645.67	2642.91	2648.41	0.004527	13.59	5692.51	741.03	0.66
Reach 1	8792	PF 1	69000.00	2624.80	2645.20	2642.34	2647.29	0.003657	12.65	6921.02	929.59	0.60

